Earthquake Engineering and the Alaskan Way Viaduct

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Principal Types of Earthquake Damage

Structural

Caused by excessive ground shaking Strongly influenced by local soil conditions

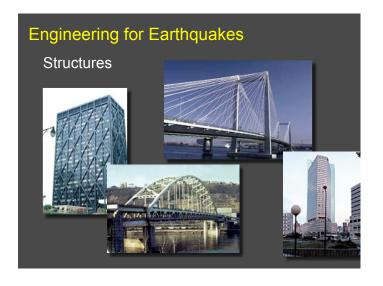
Geotechnical

Caused by ground failure Strongly influenced by local soil conditions





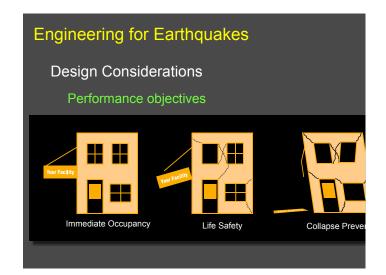


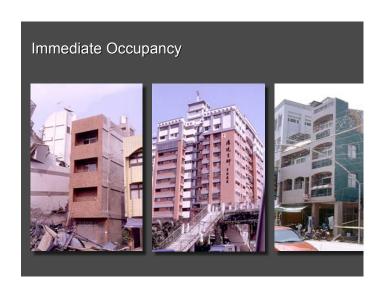


Engineering for Earthquakes

Structural Engineering Considerations

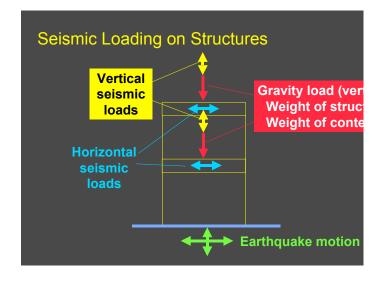
- Design of new structures
- Retrofitting of existing structures

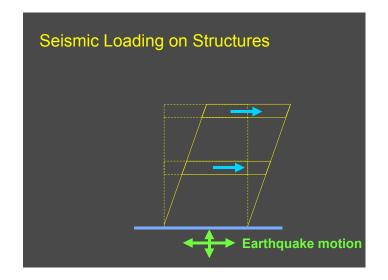


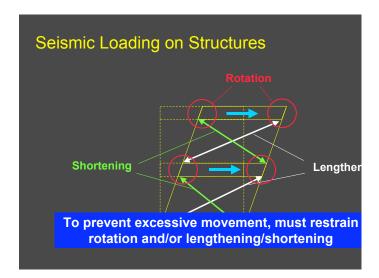


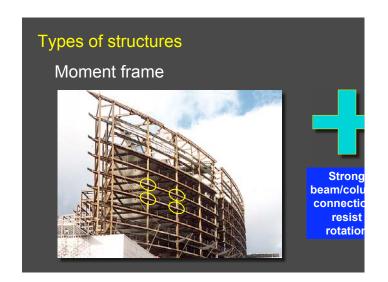


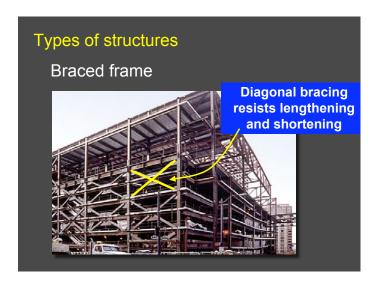


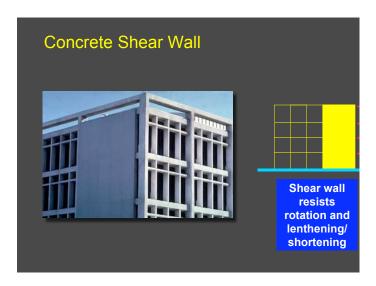




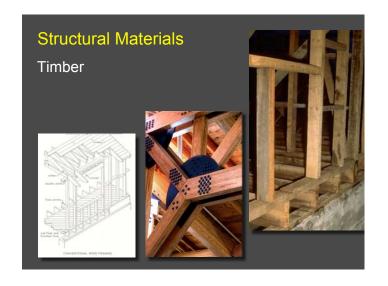


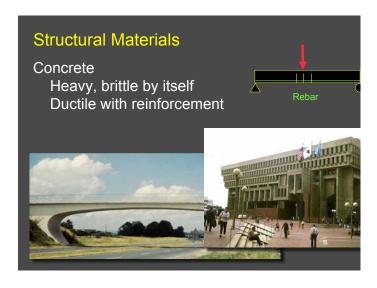










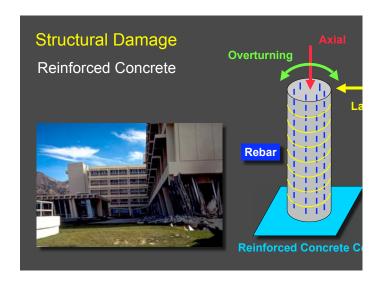








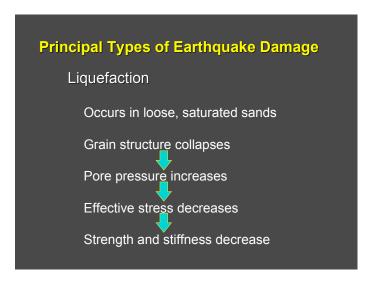










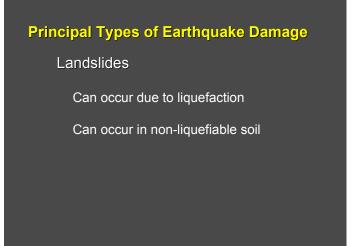




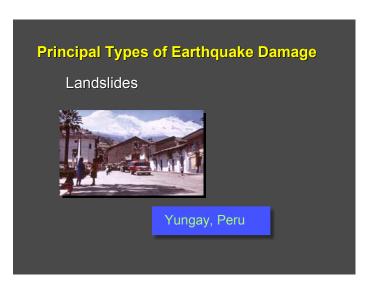








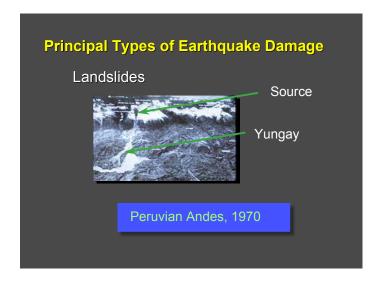


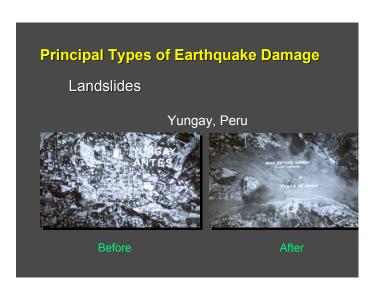


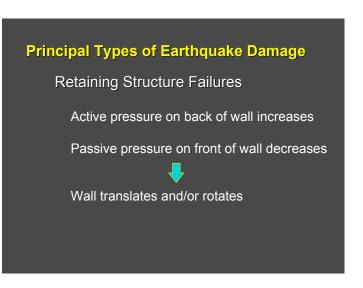








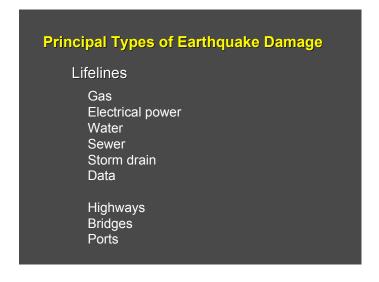


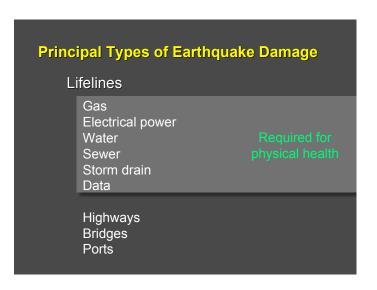


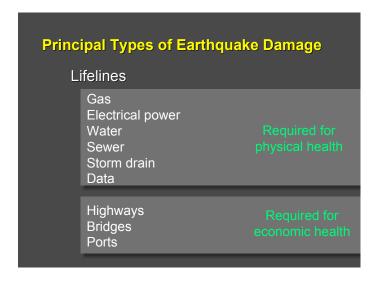












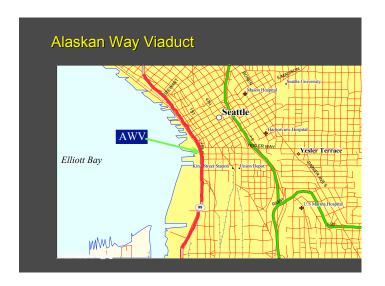


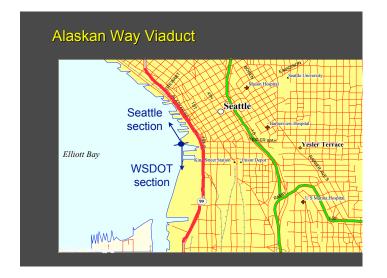






Alaskan Way Viaduct • 2.2 miles long • 86,000 vehicles per day • North of Yesler Designed by City of Seattle Constructed in 1950 • South of Yesler Designed by Washington State DOH Constructed in 1956













Seismic Vulnerability Concerns Loma Prieta earthquake M=7.1 100 km south of Oakland Cypress Structure Highway 17 in Oakland Double-deck reinforced concrete structure Similar age Similar design requirements Pile supported due to soft surficial soils

Cypress Structure



Cypress Structure



Alaskan Way Viaduct Investigations

• 1990 WSDOT internal review

• 1991-92 UW review

• 1993-95 UW/WSDOT investigation

• 1995-96 WSDOT seawall investigation

UW / WSDOT Investigation

- Structural Engineering Aspects
- Geotechnical Engineering Aspects

WSDOT Seawall Investigation

- Seawall performance
- Effects on AWV
- Remediation strategies

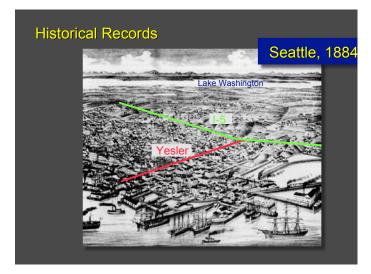
Geotechnical Engineering Investigation

- Site characterization
- Seismic hazard analysis
- Ground response analyses
- Foundation response characteristics
- Evaluation of liquefaction hazards

Site Characterization

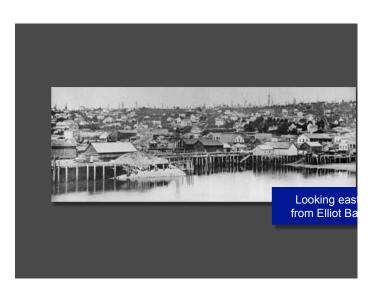
- Review of historical records
- Review of previous subsurface investigations
- Supplemental subsurface investigations
 - SPT
 - CPT
 - Seismic cone
 - Downhole seismic

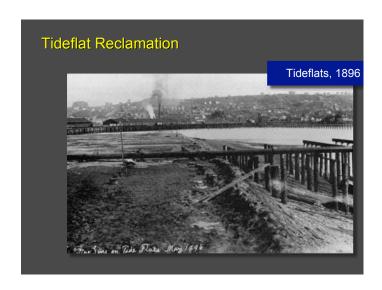




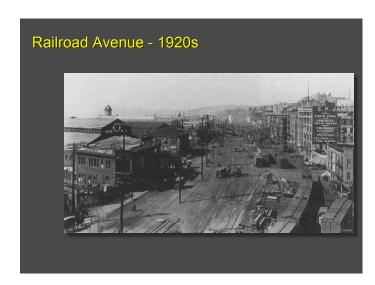


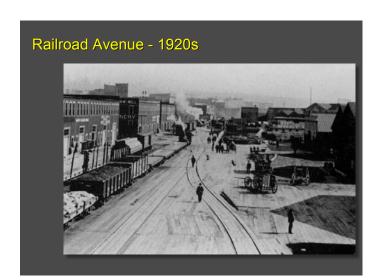


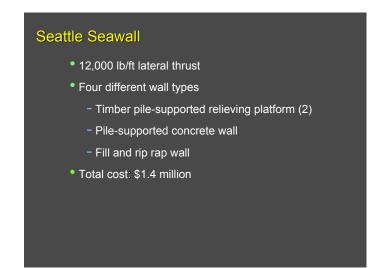


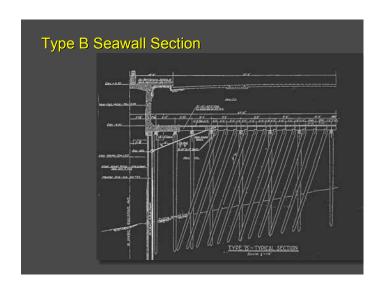


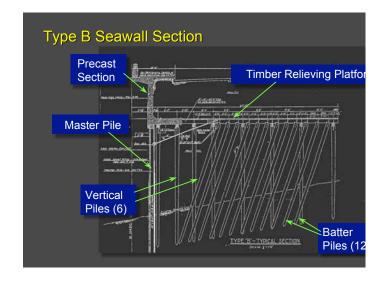


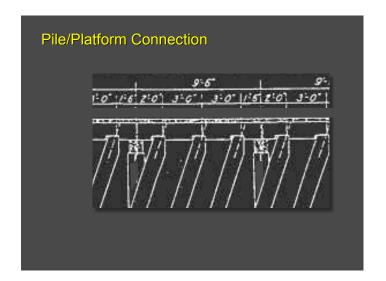


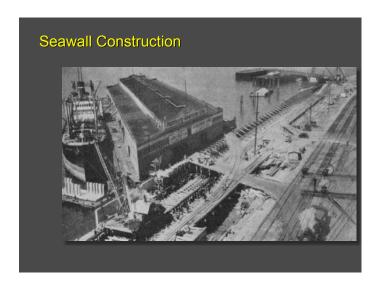


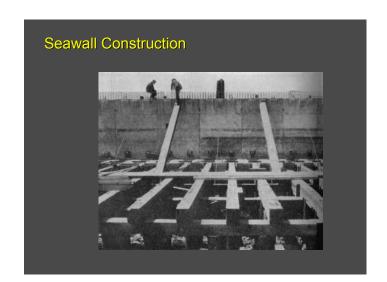


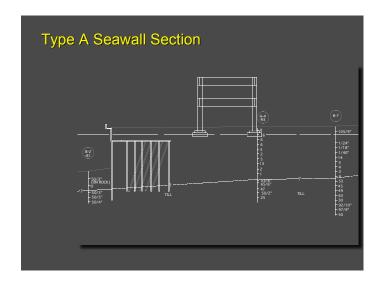


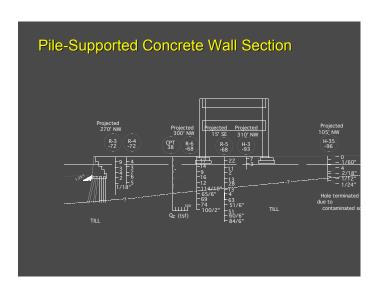


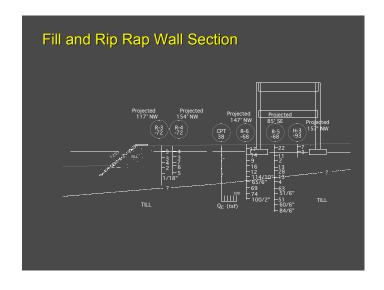




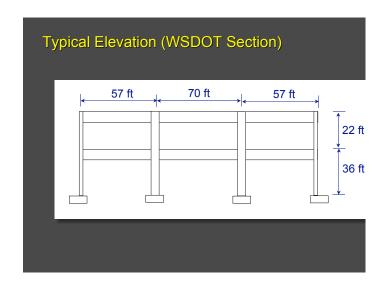


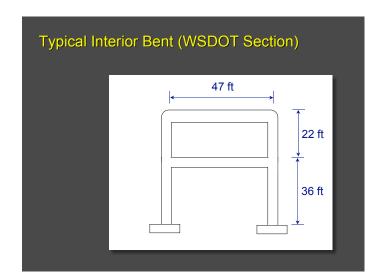


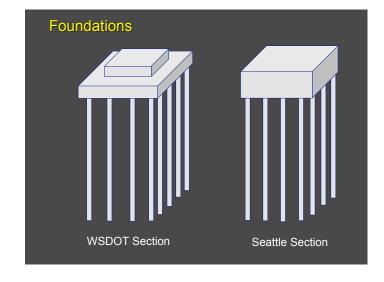


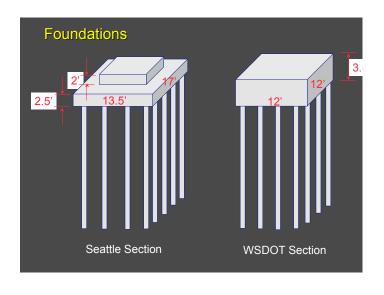


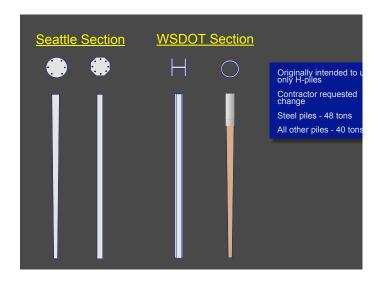
Alaskan Way Viaduct History Originally intended as downtown bypass Design began in 1948, bids opened 1949 Seattle section opened April 4, 1953 WSDOT section opened Sept 3, 1959 Seneca Street off-ramp opened 1961 Columbia Street on-ramp opened 1966 Facts 7,600 ft long 58,867 yards of concrete, 7,460 tons of rebar 171,410 ft of piling

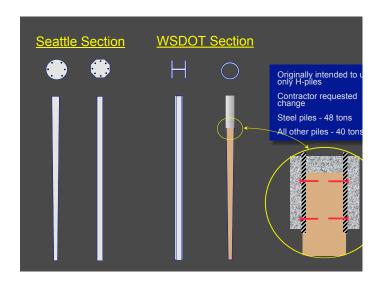


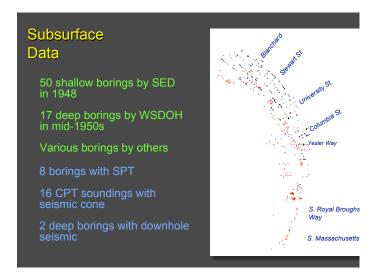


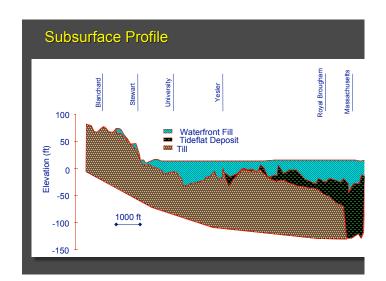


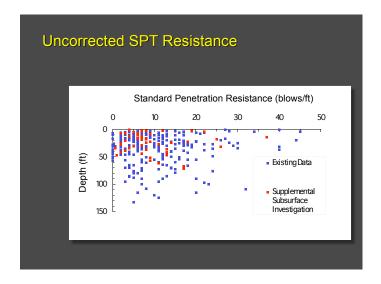


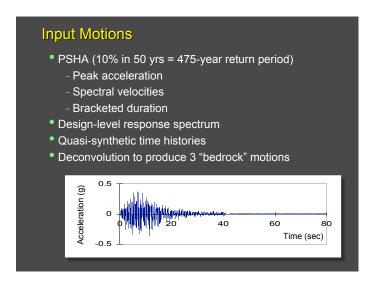


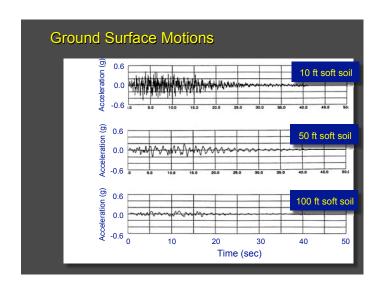






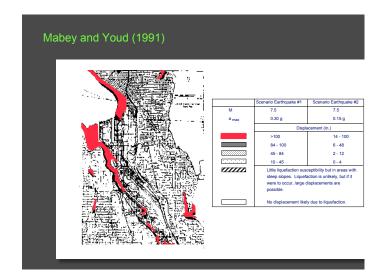


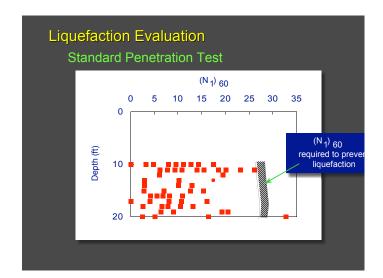


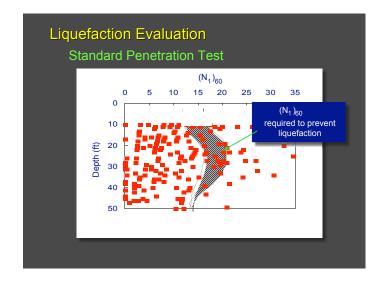


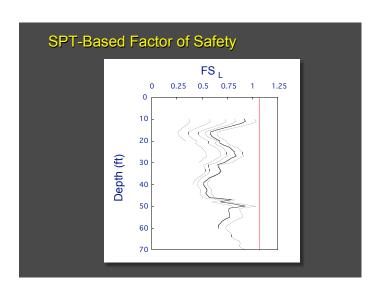
Liquefaction Susceptibility

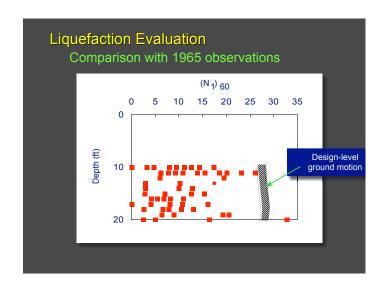
- Historical evidence
 - Sand boils in 1949 and 1965
 - Broken pipes in 1949 and 1965
 - Lateral movements in 1965
- Construction techniques
 - Hydraulic filling
 - Dumping through water
- Previous investigations
 - Mabey and Youd (1991)
 - Grant et al. (1992)

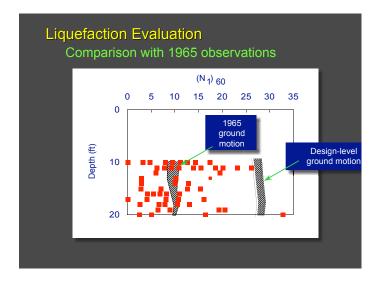


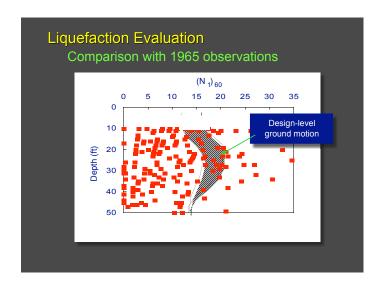


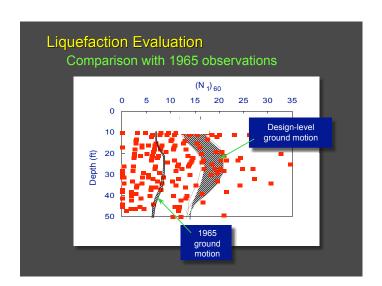


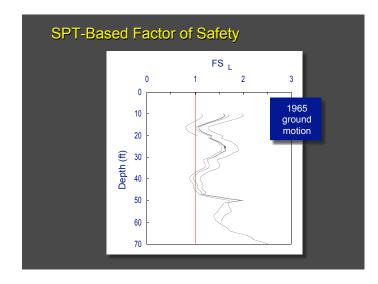












Effects of Liquefaction

- Sand boils expected over most of length
- Post-earthquake settlement
 - Up to 1" in fill above water table
 - Up to 25" in soft, saturated soils
- Vertical pile movement
 - Tip capacity reached at r_{..} = 0.6
 - Tips of southernmost piles in liquefiable soil
- Lateral pile movement
 - Depends on lateral soil movement
 - 10"-12" expected to cause bending failure
 - Lateral soil movement depends on seawall movement

All movements variable due to variability of soil profile

Seawall Investigation

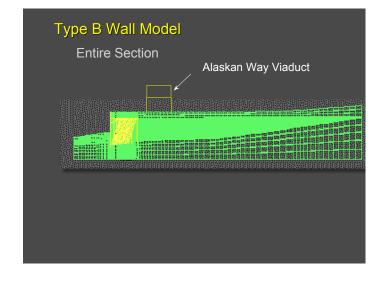
Estimation of permanent deformations due to liquefaction

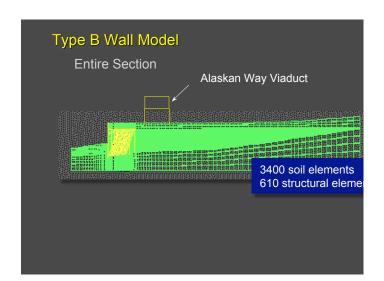
- Transverse profile characterization
 - 5 additional borings (2 offshore)
 - 3 additional CPT soundings
- Seawall structure characterization
 - Member sizes
 - Member properties
 - Connection strengths
- Computational model
 - Soil
 - Seawall
 - Soil-seawall interaction

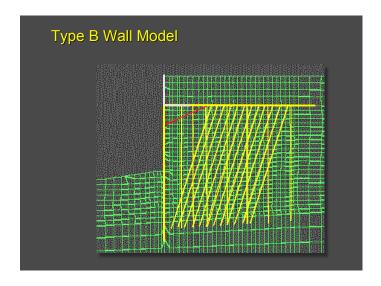


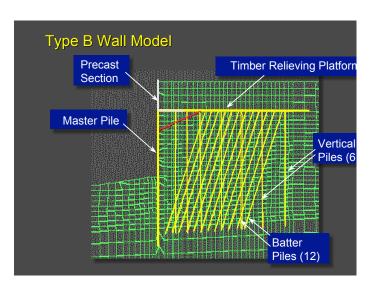
FLAC Fast Lagrangian Analysis of Continua • Explicit finite difference code • Large-strain capabilities • Several soil constitutive models • Structural elements (beams, piles, cables) • Interface elements (normal and shear) • Coupled stress-deformation and flow capabilities • Incremental construction modeling • Graphical display of results • Dynamic option • Creep option

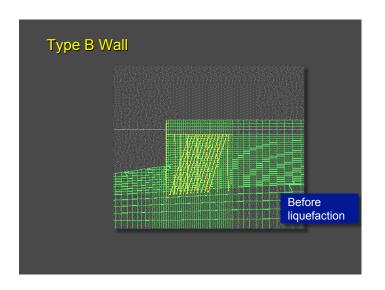
• FISH programming language

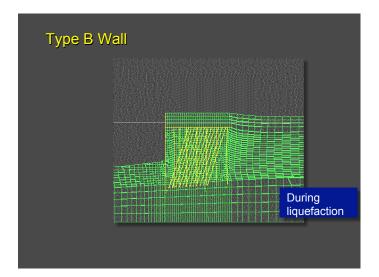


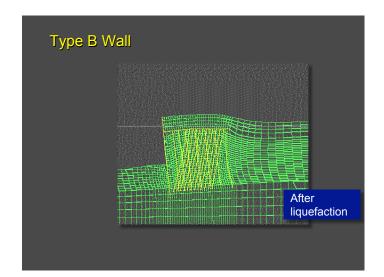


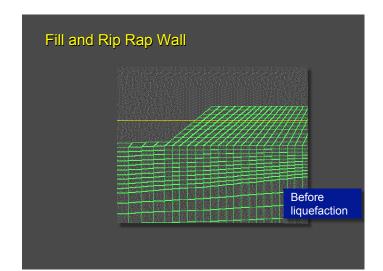


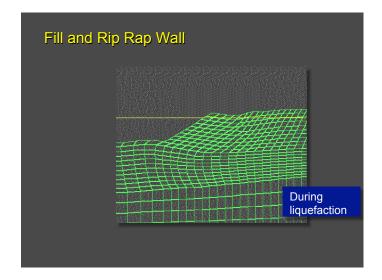


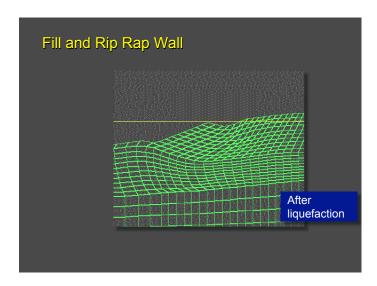


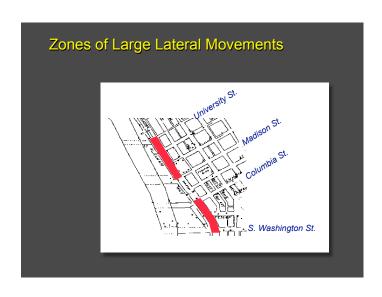




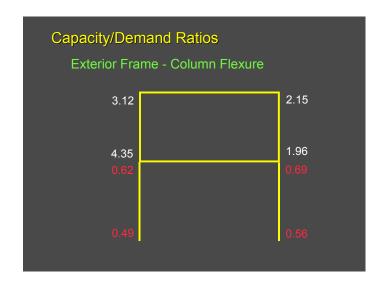


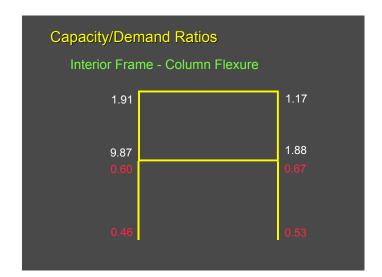


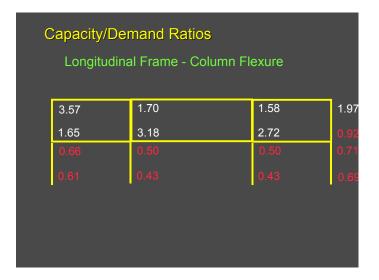


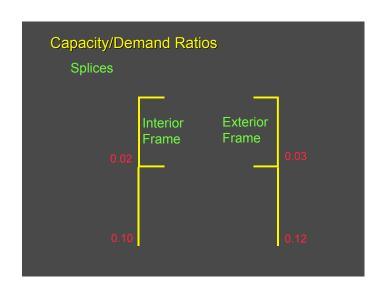


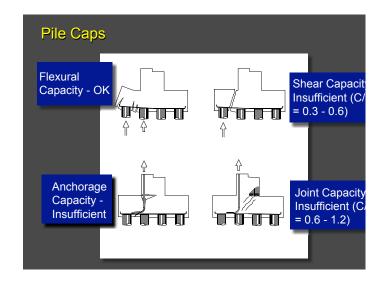
Structural Aspects of Seismic Vulnerability Dynamic Response Spectrum Analyses Nonlinear "Pushover" Analyses Investigated capacities and demands for: Flexure (beams and columns) Shear (beams and columns) Splices Joints Pile Caps











Summary of Structural Vulnerability

- Lower-level splices highly vulnerable
- Joints highly vulnerable
- Columns shear capacity marginal
- Footings vulnerable to brittle failure
- Special sections require additional investigation
 - Outrigger bents
 - On/off ramp sections

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Effects of liquefaction-induced lateral soil movements will dominate effects of shaking